



XXXVIII International School of Hydraulics, Lack, Poland



A presentation on

Discharge Characteristics of Triangular Weir with Upstream Ramp and its CFD Modelling using Ansys CFX Module

Presented by

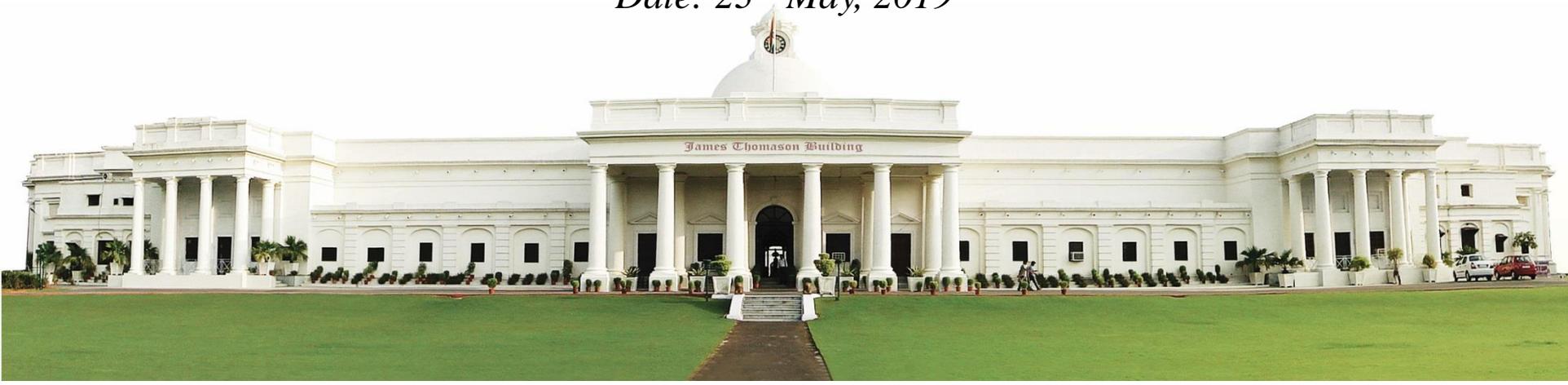
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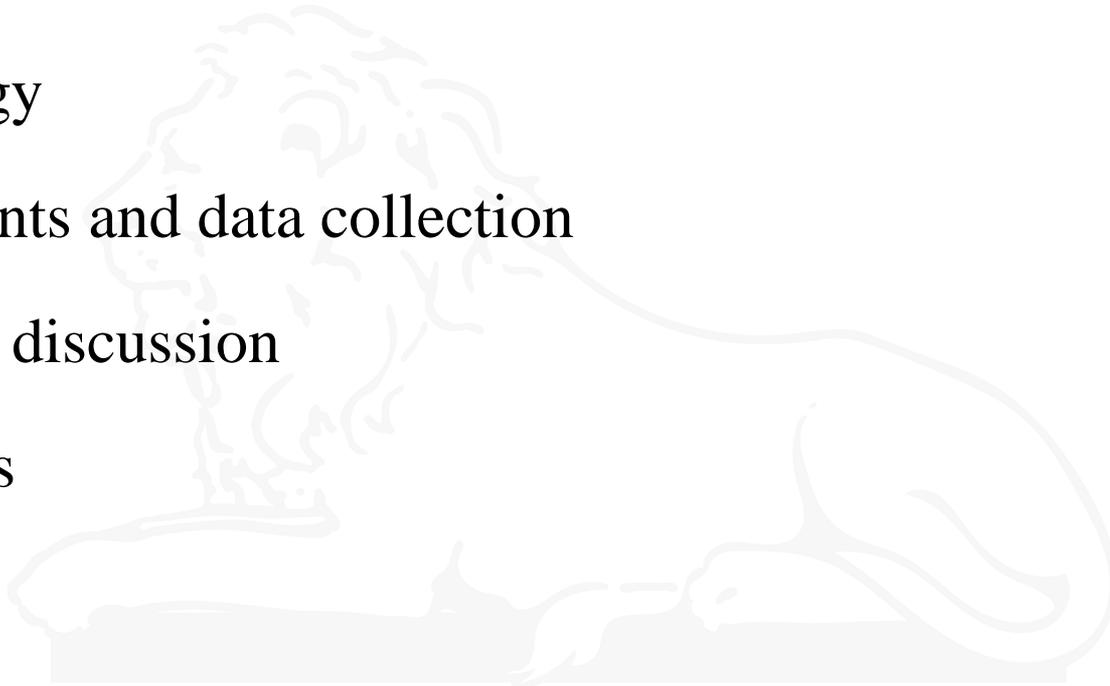
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Date: 23rd May, 2019



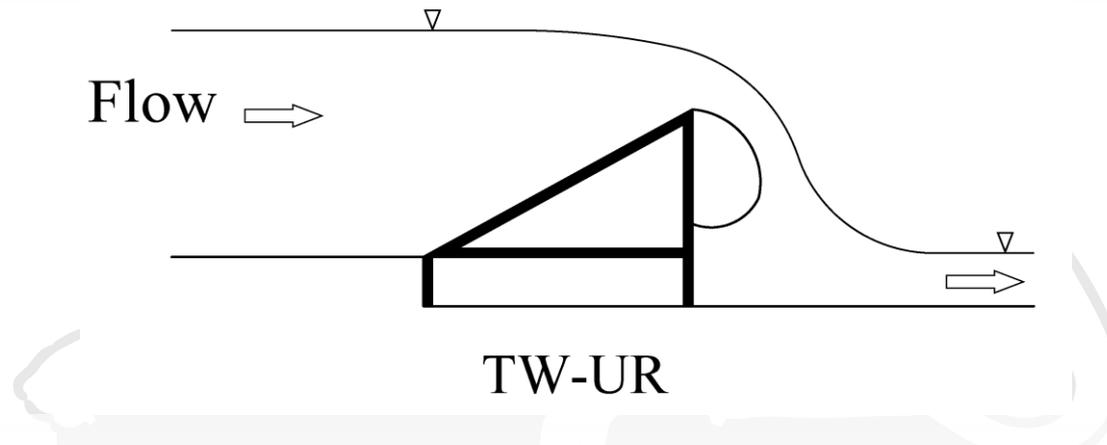
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Introduction

- A transverse hydraulic structure like a weir affects the upstream flow condition and destabilises the sediment continuity (Bai and Duan 2014).
- The restriction in sediment passage results in deposition in the upstream of a weir and may scour in the downstream side (Kim et al. 2014).



- A triangular weir with an upstream ramp (TW-UR) or a Piano Key Weir (PKW) has advantage of an adverse upstream bed than a sharp-crested weir (SCW) or broad crested weir (BCW).

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- According to Azimi and Rajaratnam (2009), weirs are divided into two groups: sharp-crested weirs (SCWs) and weirs of finite crest length.
- A TW-UR falls under the second group.
- The coefficient of discharge (C_d) for a TW-UR can be evaluated using the following basic equation used for a free flowing weir

$$Q = \frac{2}{3} C_d B \sqrt{2gH^3} \quad (1)$$

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- Initially, Azimi et al. (2013) suggested Eq. (2), and later, Di Stefano et al. (2016) proposed Eq. (3) for the estimation of C_d .

$$C_d = 1.27 \left(\frac{H}{L} \right)^{0.11} / \left(\sqrt{3} \left(\frac{P}{L} \right)^{1/10} \right) \quad (2)$$

$$C_d = 1.058 \left(\frac{H}{P} \right)^{0.0839} \left(\frac{0.002}{P} \right)^{-0.0264} \left(1 + \frac{L}{P} \right)^{0.1134} / \sqrt{3} \quad (3)$$

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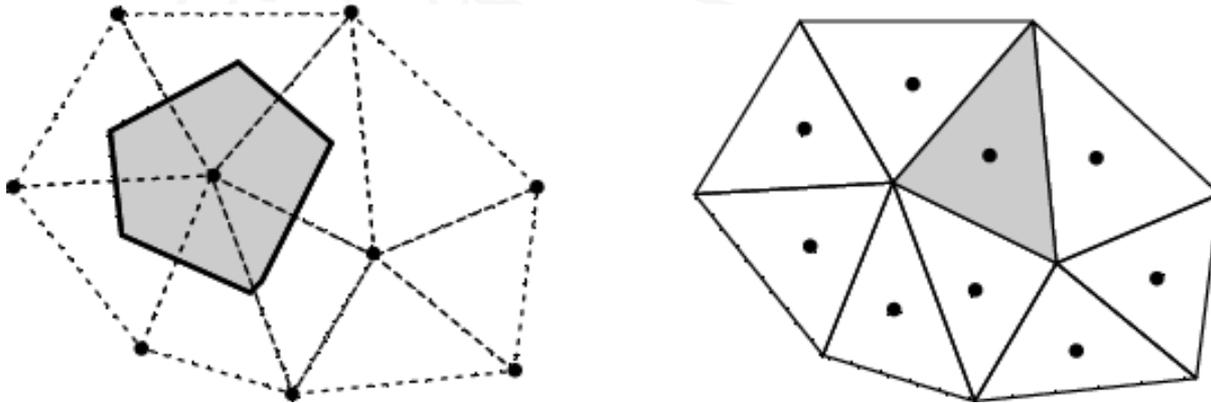
- The coefficient of discharge for a SCW ($C_{d,sharp}$) is calculated using the Rehbock equation (Henderson 1966), Eq. (4).

$$C_{d,sharp} = 0.611 + 0.08 \frac{H}{P} \quad (4)$$

- The Computational Fluid Dynamics (CFD) simulation was used earlier for different weirs and it was found to be handy, accurate, time and cost saving tool.
- The present CFD simulation is on a TW-UR using Ansys CFX within Ansys 19.1 academic research version (ANSYS 2018).

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- Ansys CFX uses the finite volume and vertex-centered methods, whereas Fluent uses the finite volume and cell-centered methods (Acharya 2016; Berggren et al. 2009).
- The vertex-centered method is useful in reducing the computational space and cost due to a less number of degrees of freedom than the cell-centered method.

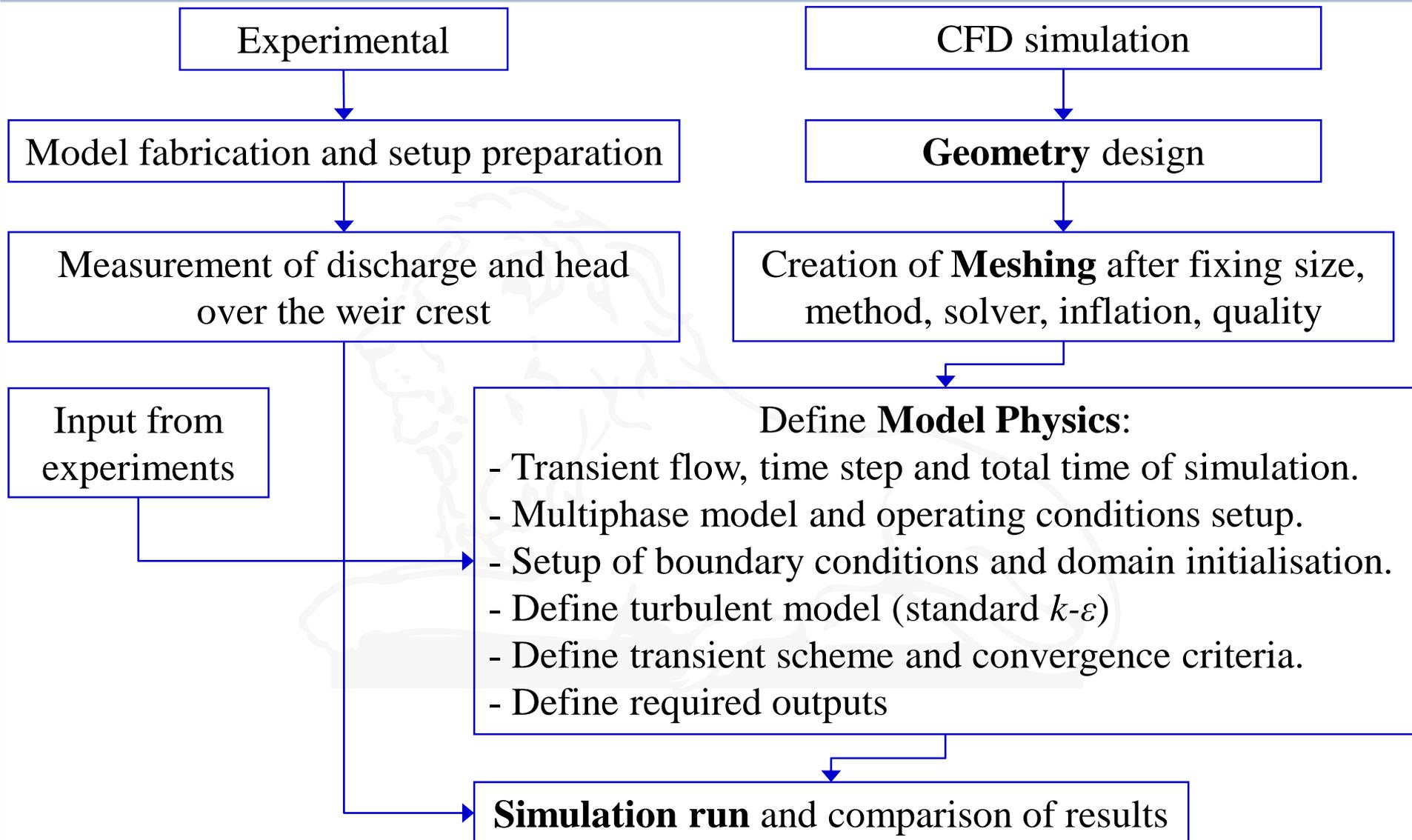


Vertex-centered vs cell-centered (Acharya 2016)

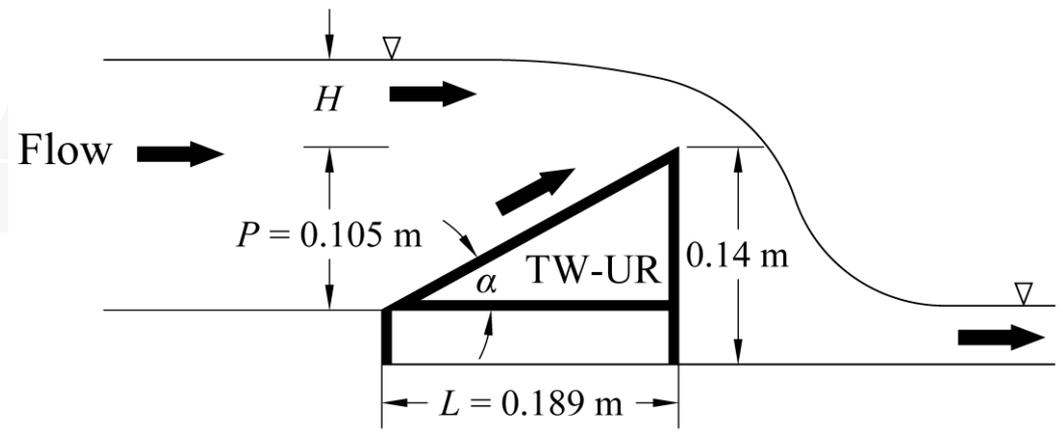
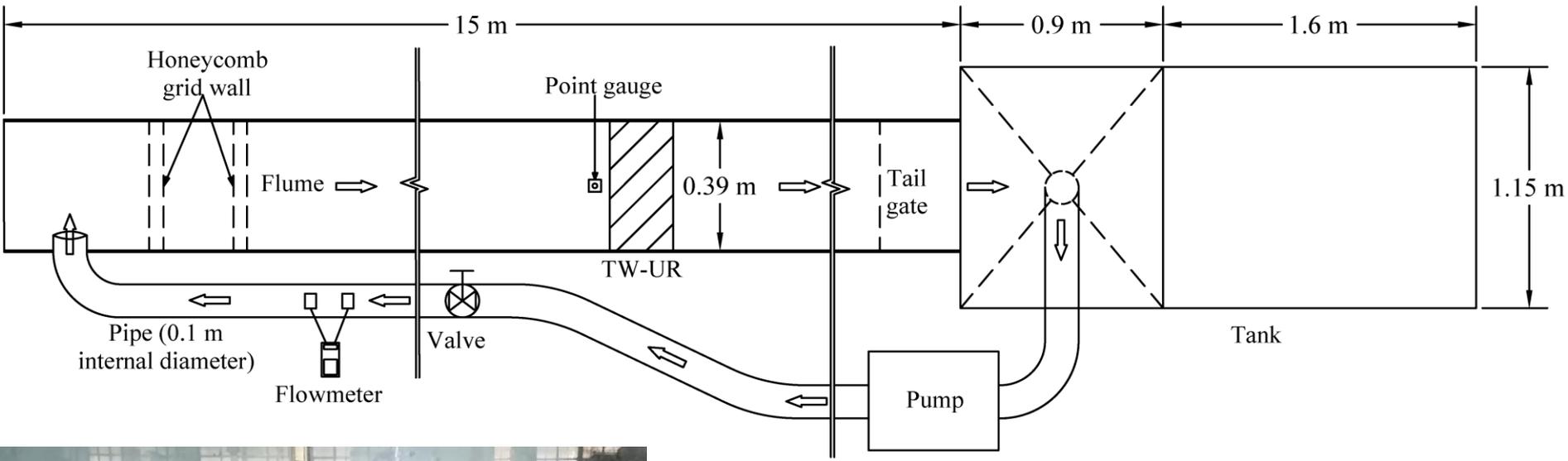
Objectives

- 1. To study the head-discharge characteristics of a TW-UR through experimentation and CFD analysis.**
- 2. Comparison between the flow fields obtained for TW-UR and SCW.**
- 3. Checking the accuracy of the existing equations of the coefficient of discharge for TW-UR.**

Methodology

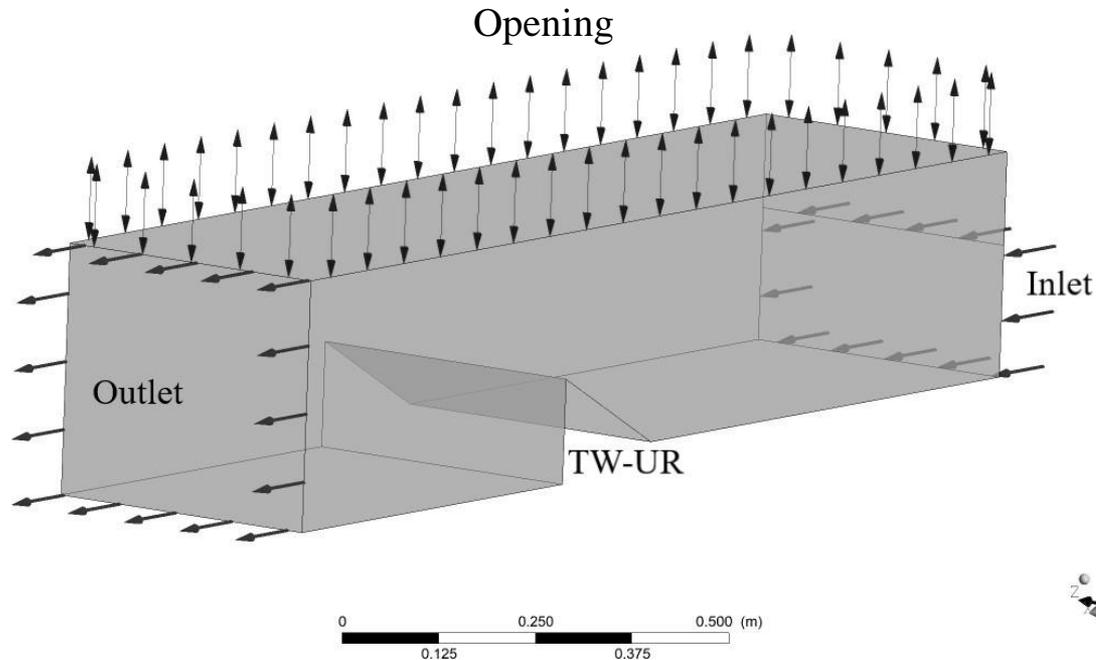


Experimental setup



CFD simulation: boundary conditions

ANSYS
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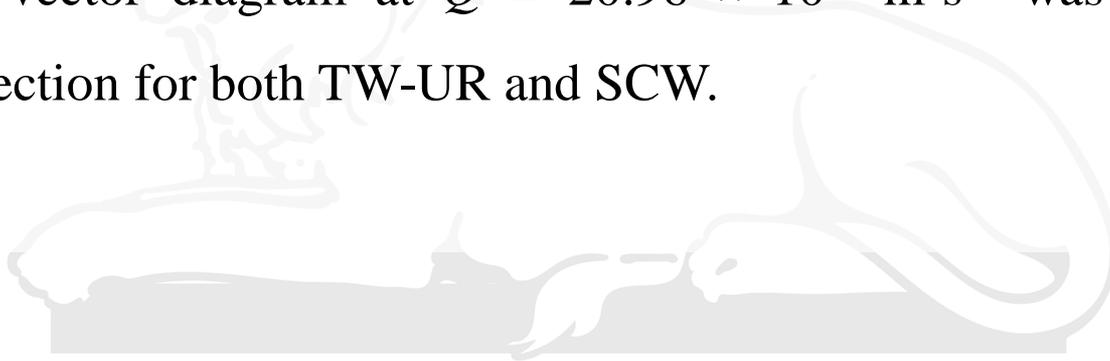


Boundary conditions

- **Inlet:** Mean velocity
- **Outlet:** Pressure based
- **Opening:** Relative pressure normal to the plane

Measurements and data collection

- Measurement of H at 0.2 m upstream of the weir crest for 23 discharges ranging from $7.3 \times 10^{-3} \text{ m}^3\text{s}^{-1}$ to $29.95 \times 10^{-3} \text{ m}^3\text{s}^{-1}$.
- The simulation for TW-UR was performed for 5 discharges out of them. The free surface level was extracted by defining an Iso-clip having water volume fraction = 0.5.
- The velocity vector diagram at $Q = 20.96 \times 10^{-3} \text{ m}^3\text{s}^{-1}$ was obtained along a longitudinal section for both TW-UR and SCW.



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- A total of 200 experimental datasets were collected to check the accuracy of the existing equations suggested by Azimi et al. (2013) and Di Stefano et al. (2016).

Table 1 Range of parameters for the present and previous investigations

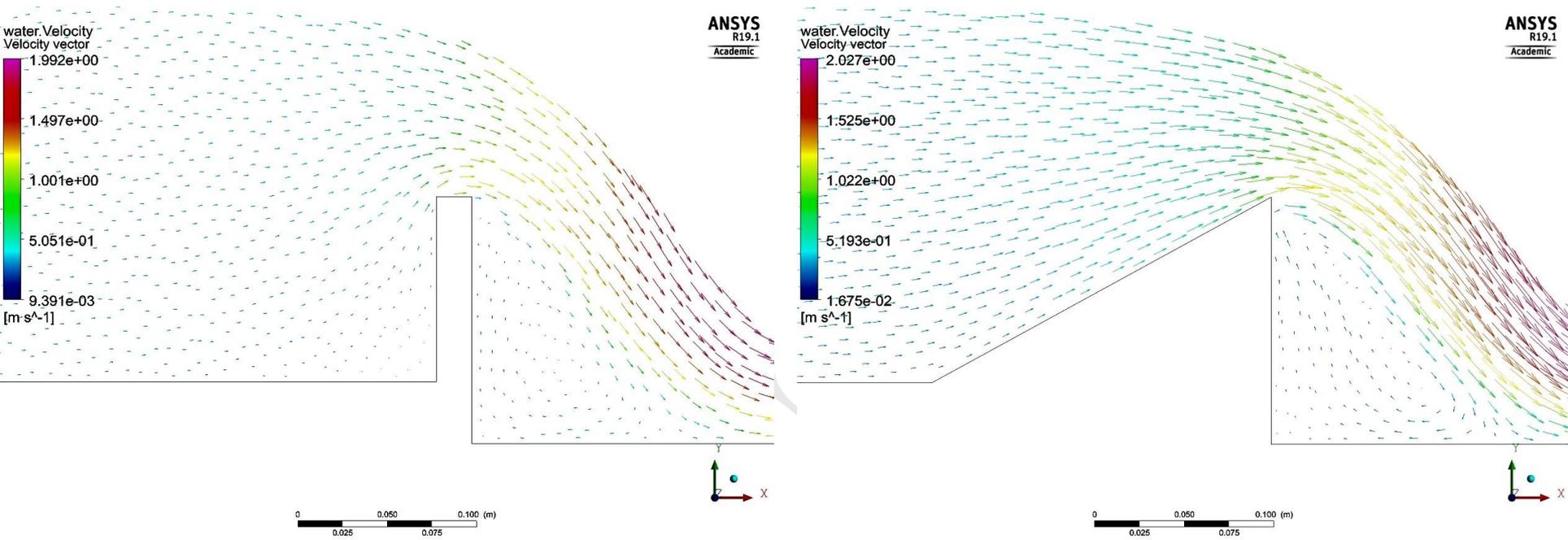
| Investigator | L [m] | P [m] | H [m] | Angle α |
|---|---------------------------|---------------------------|-----------------|-----------------------------|
| Abou-Seida and Quraishi (1976) (# group 2 case 1) | 0.1415, 0.117, 0.0882, | 0.0818, 0.117, 0.1527 | 0.0335 – 0.1273 | 30°, 45°, 60° |
| Bazin (# 125) (from Horton 1907) | 0.167 | 0.50 | 0.30 – 0.427 | 71.6° |
| Shaker and Sarhan (2017) | 0.06 – 0.30 | 0.06, 0.08, 0.10, 0.12 | 0.021 – 0.06 | 21.8°, 26.6°, 33.7°, 45° |
| USDW (# 16) (from Horton 1907) | 5.63 | 1.45 | 0.51 – 1.27 | 15.9° |
| Present study | 0.189 | 0.105 | 0.0425 – 0.1055 | 29.1° |

indicates the number/set of the experiment.



Results and Discussion

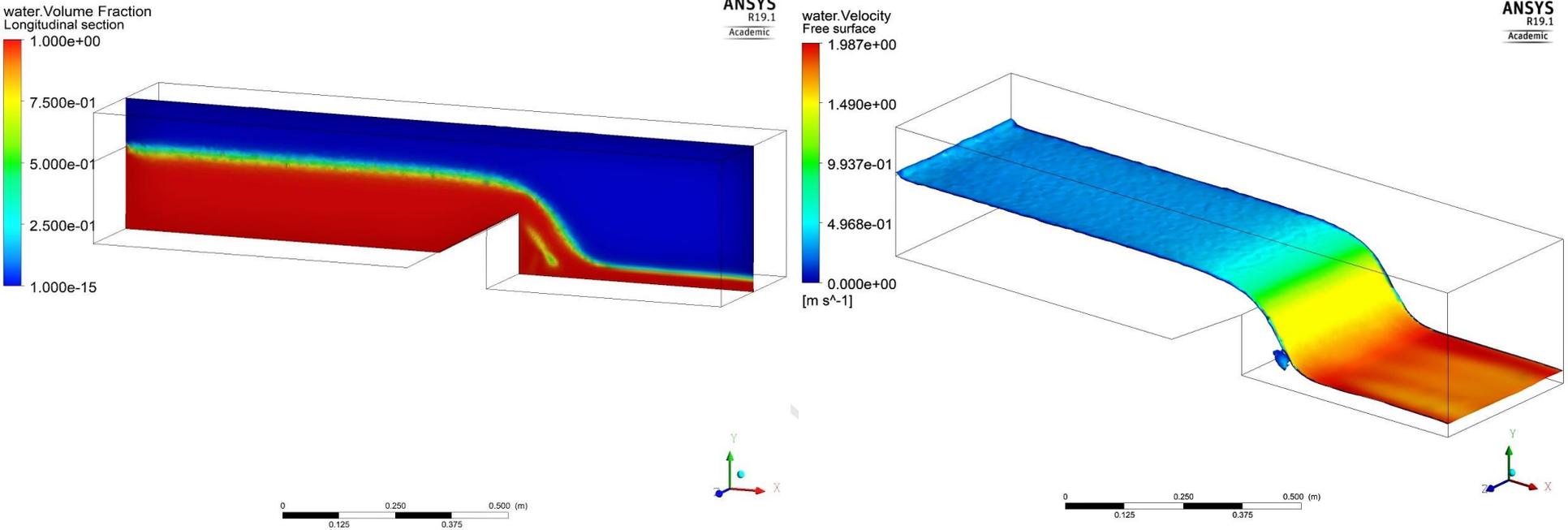
Flow field obtained from CFD simulation



Flow field for a SCW and TW-UR at $20.76 \times 10^{-3} \text{ m}^3\text{s}^{-1}$ discharge

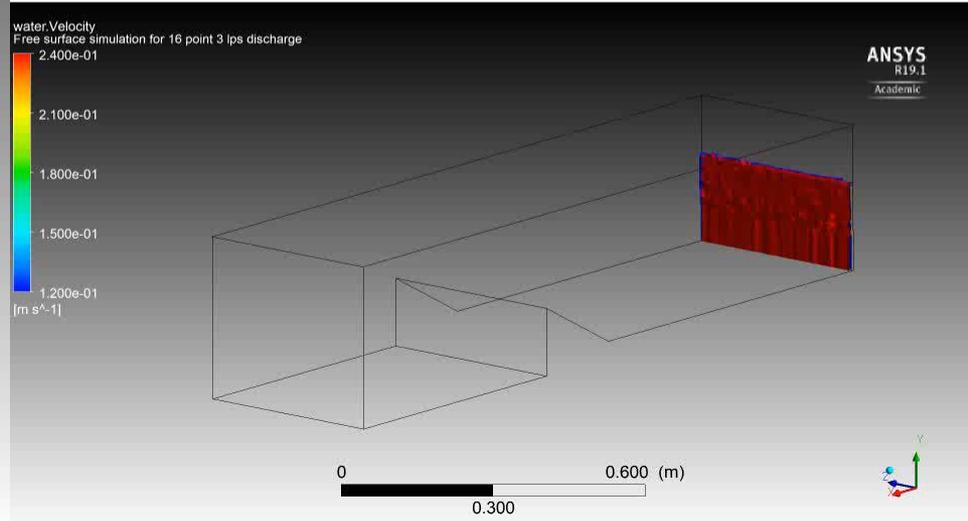
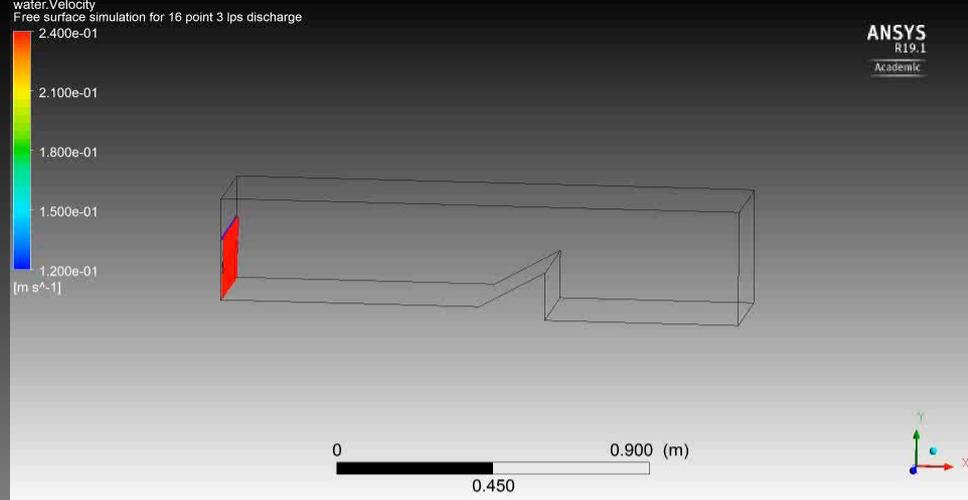
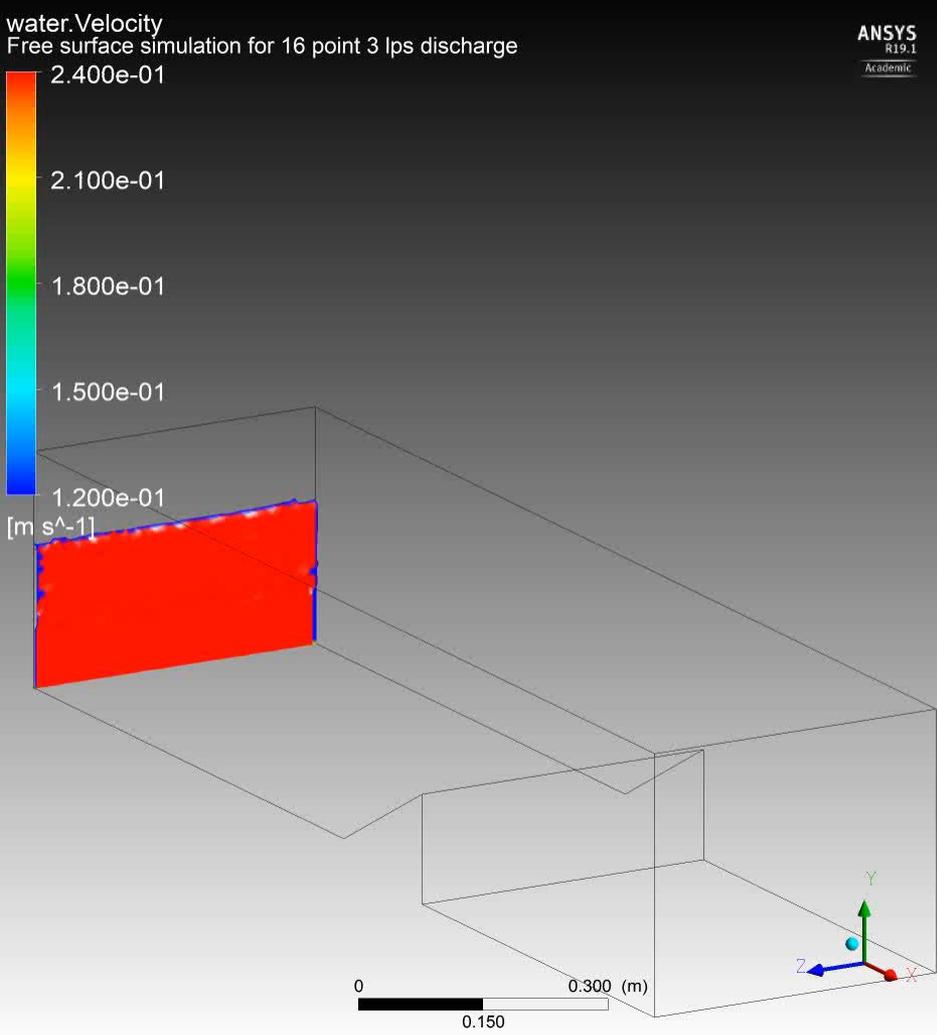
- TW-UR has more active flow field as compared to a SCW.
- Velocity increased along the flow due to flow contraction.
- Increases discharge capacity and possibility of sediment passage.

Air-water mixture and free surface

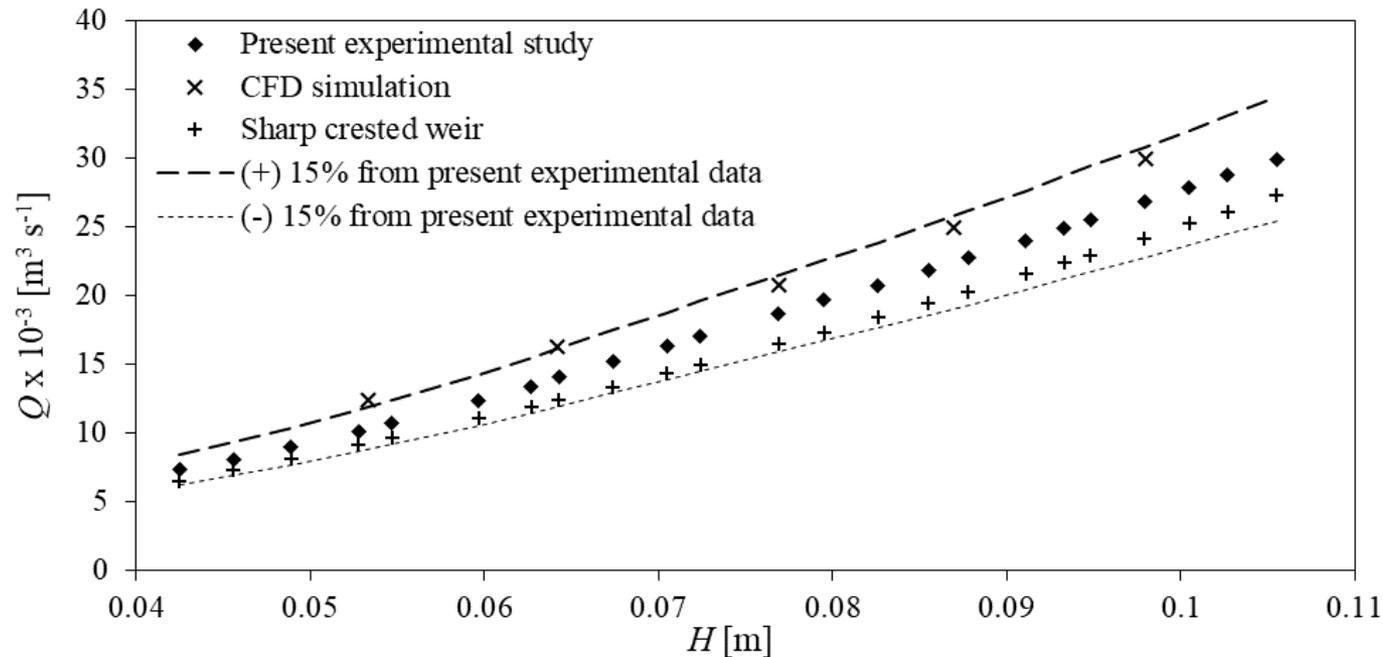


**Simulated flow for TW-UR at $20.76 \times 10^{-3} \text{ m}^3 \text{ s}^{-1}$ discharge:
air-water mixture and free surface**

Simulated flow



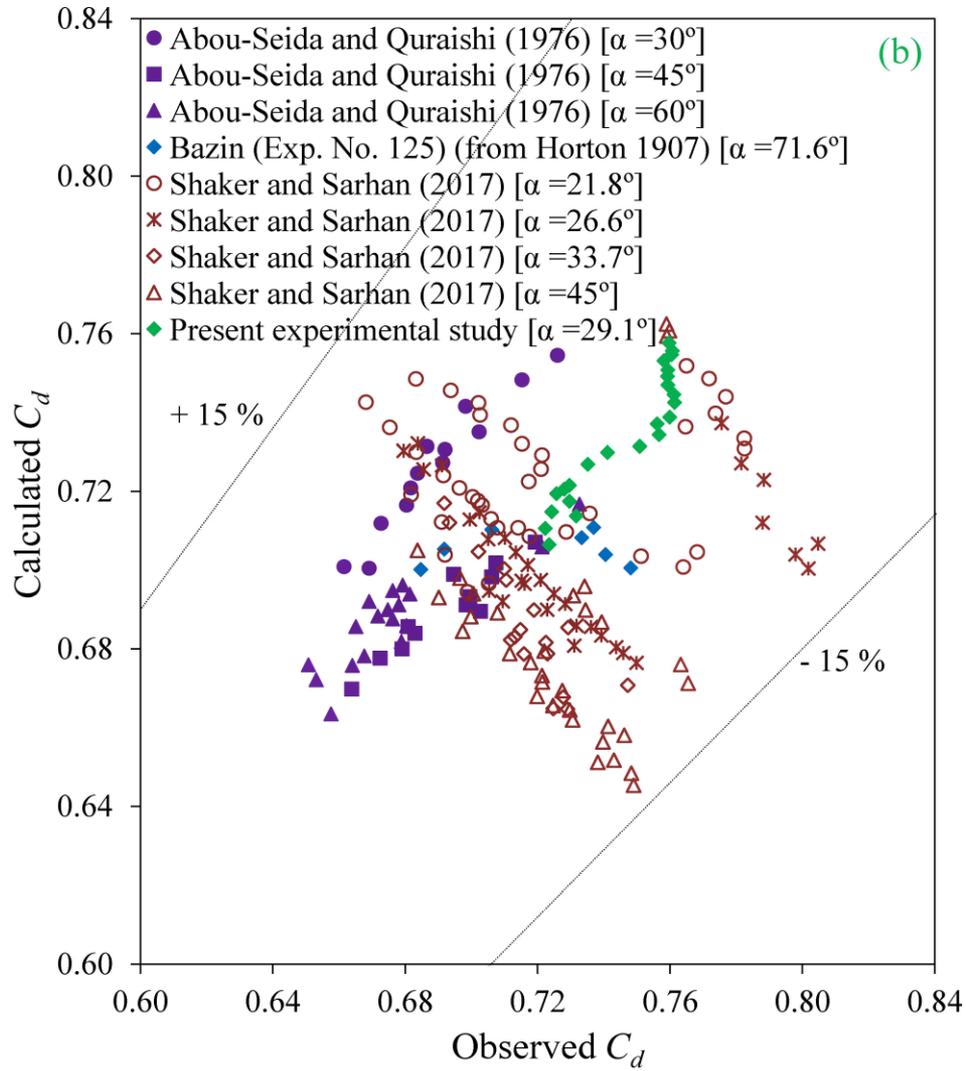
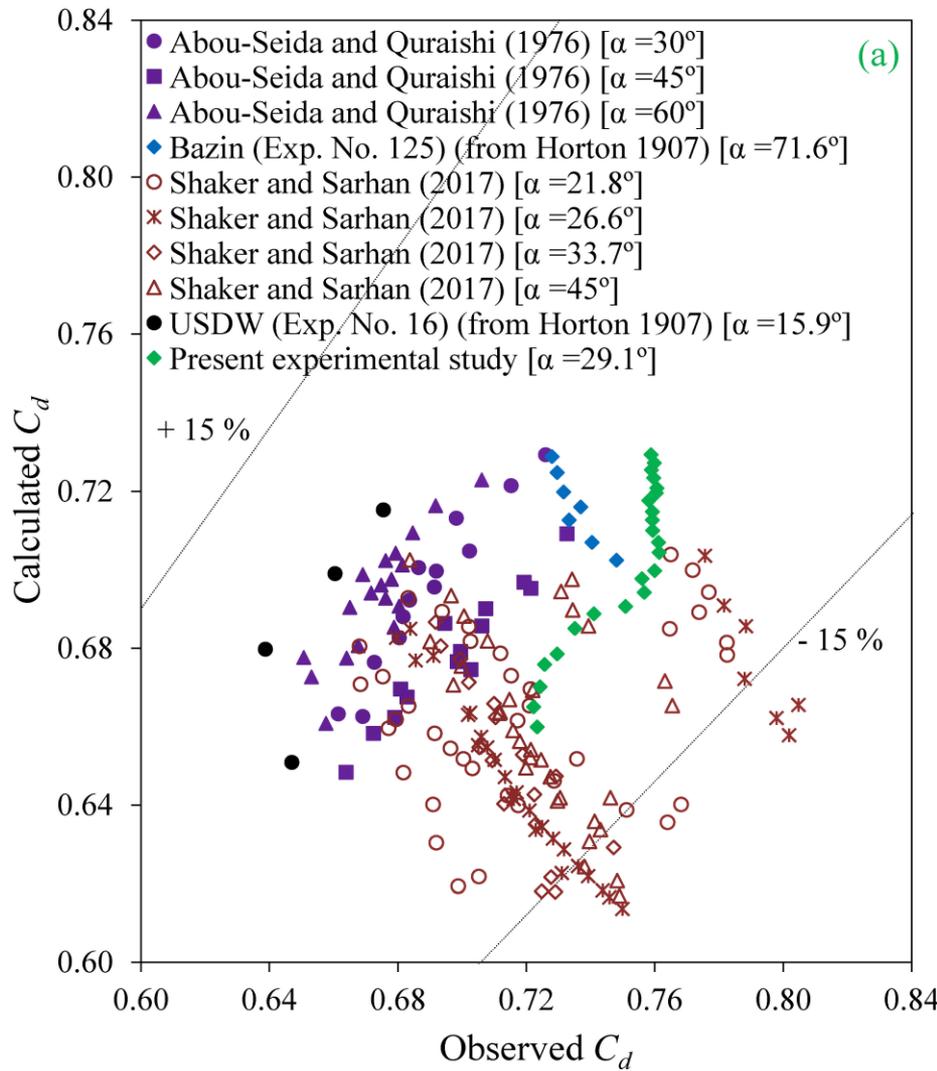
Comparison of the discharge obtained from CFD with the observed value



Head-discharge correlation

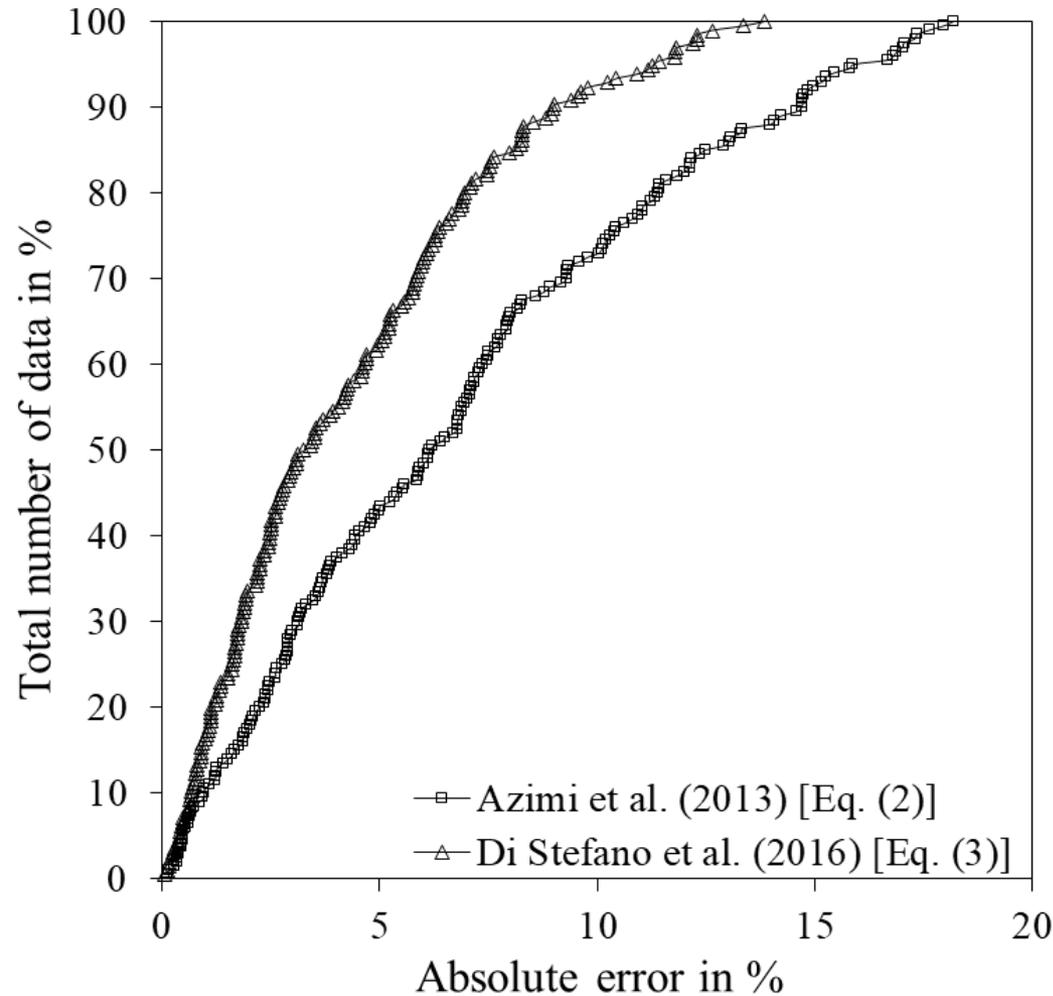
- TW-UR had about 9.8% to 14.3% higher discharging capacity than SCW.
- For TW-UR, CFD simulation estimated about 10% to 15% higher discharge under the same head in comparison to the observed results.
- C_d increased initially with H , but remained almost constant beyond $H/P \approx 0.65$

Checking the accuracy of existing equations for C_d



Calculated C_d vs observed C_d for: (a) Azimi et al. (2013), (b) Di Stefano et al. (2016)

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Total number of data vs absolute error

- The maximum absolute error for Eqs. (2)-(3) was 18.2% and 13.8%, respectively.
- The total number of datasets lay within 5%, 10% and 15% absolute error ranges for Eq. (2) was 43.0%, 72.5% and 92.5%, and for Eq. (3) it was 61.7%, 92.3% and 100%, respectively.

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- The equations were also evaluated based on two statistical parameters; mean absolute percentage error (*MAPE*) and root mean square error (*RMSE*) as suggested by Aydin and Emiroglu (2013); Aydin and Emiroglu (2016); Crookston et al. (2018):

$$MAPE = \frac{1}{N} \sum_{i=1}^N \left| \frac{C_{do} - C_{dc}}{C_{do}} \right| \times 100\%$$

$$RMSE = \sqrt{\frac{1}{N} \sum_{i=1}^N (C_{do} - C_{dc})^2}$$

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Table 2 Error in C_d prediction for Eq. (2)-(3)

| Investigator | Angle α | Eq. (2) | | Eq. (3) | |
|---|----------------|---------------------|-------------|--------------------------|-------------|
| | | (Azimi et al. 2013) | | (Di Stefano et al. 2016) | |
| | | RMSE | MAPE (%) | RMSE | MAPE (%) |
| Abou-Seida and Quraishi (1976) (# group 2 case 1) | 30° | 0.008 | 0.91 | 0.037 | 5.40 |
| | 45° | 0.019 | 2.67 | 0.009 | 1.07 |
| | 60° | 0.020 | 2.84 | 0.015 | 2.05 |
| Bazin (# 125) (from Horton 1907) | 71.6° | 0.025 | 2.67 | 0.028 | 3.37 |
| Shaker and Sarhan (2017) | 21.8° | 0.066 | 7.59 | 0.039 | 4.57 |
| | 26.6° | 0.091 | 10.86 | 0.050 | 5.58 |
| | 33.7° | 0.073 | 8.98 | 0.039 | 4.55 |
| | 45° | 0.074 | 8.84 | 0.059 | 6.92 |
| USDW (# 16) (from Horton 1907) | 15.9° | 0.034 | 4.68 | — | — |
| Present experimental study | 29.1° | 0.050 | 6.54 | 0.012 | 1.41 |
| Total data | - | 0.062 | 6.86 | 0.04 | 4.30 |

Conclusions

- A TW-UR model has higher discharging capacity than a SCW of same height (about 9.8% to 14.3% higher discharge was observed in the present study).
- The ramp and a highly active flow field in upstream of TW-UR are enhancing its discharging capacity and sediment passage capability than SCW and BCW.
- CFD simulation estimated about 10% to 15% higher discharge than observed value.
- Both graphical and statistical analysis has shown that the equation of C_d proposed by Di Stefano et al. (2016) is more accurate than the equation proposed by Azimi et al. (2013).





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THANK YOU

